

Breach # 1: Elev 10.00 ft, Load # 8
Load Combination # 8, D+W 0 Deg

Controlling: Vertical Stiffener, Circumferential Stiffener, Shell Buckling Capacity w/ Breach

Breach Analysis based upon 'Tubular Steel Structures' by M.S. Troitsky
Analysis assumes that this is the only breach and doesn't consider interaction with other openings.

Elev	= Elevation Where Breach Ctr Line Intersects Stack Ctr Line	= 10.000 ft
ElTop	= Elevation at Top of Breach	= 14.000 ft
ElBot	= Elevation at Bottom of Breach	= 6.000 ft
Wid	= Width of Breach Opening	= 60.0000 in
Ht	= Height of Breach Opening	= 96.0000 in
Tilt	= Tilt of Duct relative to grade (0 is Horizontal)	= 0.0 Deg
OD	= Outer Diameter at Elevation	= 186.0000 in
Thk	= Thickness of Stack	= 0.5000 in
Loads Acting @ Breach (Take worst of loads at Top, Mid and Bot of Breach):		
V	= Vertical Load	= 114.77 Kip
M	= Moment due to Lateral Loads	= 2097.69 k-ft
Yc	= Distance from Centerline to Elastic Neutral Axis	= 26.8174 in
r	= Radius to Outside of Stack	= 93.0000 in
r1	= Radius to Inside of Stack	= 92.5000 in
Dm	= Mean Diameter: $2*(r+r1)/2$	= 185.5000 in
Alpha	= Half Angle of Opening: $\text{ArcSin}(Wid/OD)$	= 0.3285 rads
Beta	= Angle to Neutral Axis: $\text{ArcSin}(Yc/r)$	= 0.2925 rads
Gamma1	= $(PI/2 - Alpha + Beta)/2$	= 0.7674 rads
Gamma2	= $(PI/2 - Alpha - Beta)/2$	= 0.4749 rads
Sc	= Statical Moment Under Compression	= 9279.80 in ³
St	= Statical Moment Under Tension	= 5057.68 in ³
Ic	= Moment of Inertia of Wall Under Compression	= 775699.22 in ⁴
It	= Moment of Inertia of Wall Under Tension	= 263587.55 in ⁴
Phi	= $(Mom + Ver*Yc) / (Ic + It)$	= 27.1821
N	= $Phi * (Sc - St)$	= 114.77 Kip

Breach # 1 Ld 8 Cylinder Buckling Capacity:

This checks the reduced buckling capacity for the cylinder considering that it is weakened by the breach cutout. Reduction is based upon Troitsky Fig 5-23 for Effect of Rect Cutout on Cylindrical Buckling Capacity.

fclassic	= Classic Buckling Stress for Cylinder: $0.6*E*t/r$	= 90.65 ksi
FS	= Factor of Safety for Compression	= 0.600
rbar	= Non-dimensional parameter: $(Wid+Ht) / (4*((OD/2)*Thk)^{0.5})$	= 5.719
P/Pcl	= Actual / Classical Buckling Capacity {Fig 5-23}; Max = 0.2	= 0.186
fmax	= Max Comp Stress: $(Phi/2)*(2*Yc+(r+r1)*COS(Alpha))$	= 3.12 ksi
fallow	= Allowable Reduced Buckling Stress: $(P/Pcl)*fclassic*FS$	= 10.10 ksi
UR_sb	= Shell Buckling Unity Ratio: $fmax/fallow$	= 0.309

This check indicates that even with the breach the shell still has enough capacity to resist the applied loads. Since vertical stiffeners were provided anyway, those will be checked using the selected criteria as if the shell was failing with $Fac = 1$.

Ver Stiff (one on each side of opening):		
Country	= Country Dimensions for Shape	= American
Shape	= IBeam Strong Direction	= IBEAM
Size	= Size of Structural Member	= W10X54
Matl	= Material of Stiffener	= A-36
Temp	= Temperature considered for Material Properties	= 70.00 Deg F
Fy	= Yield Stress of Stiffener @ Temp	= 36.00 ksi
E	= Modulus of Elasticity of Stiffener @ Temp	= 29400.00 ksi
Dv	= Distance from Opening Edge to Edge of Stiffener	= 3.0000 in
Dva	= Distance from Opening Edge to Top of Stiff Above Opening	= 3.0000 in
Dvb	= Distance from Opening Edge to Top of Stiff Below Opening	= 3.0000 in
Dca	= Dist from Opening Edge to Circ Stiffener Above Opening	= 3.0000 in
Dcb	= Dist from Opening Edge to Circ Stiffener Below Opening	= 3.0000 in

Breach # 1 Ld 8 Ver Stiff: Complete Breach Area Replacement:

HA	= Half Angle of Breach Opening	= 18.82 Deg
		= 0.3285 rads
Ycent	= Dist from OD to Stiff Centroid: $D/2$	= 5.0500 in
ds	= Dist from Ctr Line to Ctr of Ver Stiff: $(OD/2+Ycent)*Cos(HA)$	= 92.8084 in
As_reqd	= Req'd Area of a Single Stiffener: $Thk*(OD/2)^2*Sin(HA)/ds$	= 15.031 sq in
Aratio	= Ratio of Required to Actual Area: As_reqd/A	= 0.951

Breach # 1 Ld 8 Ver Stiff: Complete Breach Moment of Inertia Replacement:

Iv_rot = Rotated Mom of Inertia of Stiff: $I_x \sin(HA)^2 + I_y \cos(HA)^2$ = 123.81 in⁴
 Irem = Mom of Inertia of Removed Sec: $r^3 * (HA + \sin(2*HA)/2) * Thk$ = 254897.01 in⁴
 Ireq = Req'd Mom of Inertia of Stiff about Stack Ctr Line: $Irem/2$ = 127448.50 in⁴
 Iv_tot = Act Mom of Inertia of Stiff about Stack Ctr Line: $Iv_rot + A * ds^2$ = 136215.64 in⁴
 Iratio = Ratio of Required to Actual Mom of Inertia: $Ireq/Iv_tot$ = 0.936

Breach # 1 Ld 8 Ver Stiff: Determine Axial Load and Bending on a Stiffener:

G = Axial Load Due to Stack Vertical Load: $V * \alpha / (2 * \pi)$ = 6.00 Kip
 Pw = Axial Load Due to Moment: $M * \alpha / (0.5 * \pi * Dm)$ = 28.37 Kip
 Pr = Total Axial Load on Stiffener: $G + Pw$ = 34.37 Kip
 dv = Depth of Vertical Stiffener Radially from Shell = 10.1000 in
 Lv = Effective Stiff Length: $Ht + Dca + Dcb$ = 102.0000 in
 Mry = Moment Due to Eccentricity: $Pr * (dv/2)$ = 173.59 K-in

Ver Stiff: Member properties taken from AISC 360-16

Country = Country Database for Member Dimensions = American
 Shape = IBeam Strong Direction = IBEAM
 Size = Size of Member = W10X54
 Ag = Cross Sectional Area = 15.800 sq in
 Ix = Moment of Inertia about X = 303.00 in⁴
 Iy = Moment of Inertia about Y = 103.00 in⁴
 ImaJ = Principal Major Moment of Inertia = 303.00 in⁴
 Imin = Principal Minor Moment of Inertia = 103.00 in⁴
 J = Torsional Constant = 1.82 in⁴
 Sx = Elastic Sec Modulus about X = 60.00 in³
 Sy = Elastic Sec Modulus about Y = 20.60 in³
 Zx = Plastic Sec Modulus about X = 66.60 in³
 Zy = Plastic Sec Modulus about Y = 31.30 in³
 Xc = Distance in X to Centroid = 0.0000 in
 Yc = Distance in Y to Centroid = 0.0000 in
 rx = Radius of Gyration about X = 4.3700 in
 ry = Radius of Gyration about Y = 2.5600 in

Ver Stiff: Nominal Compressive Strength per AISC 360-16 Sec E3:

Lc = Effective Length of Member = 102.0000 in
 r = Minimum of Radius of gyration: $\min(rx, ry)$ = 2.5600 in
 Lc/r = Slenderness Ratio = 39.844
 Limit = Limit used for Lc/r: $4.71 * (E/Fy)^{0.5}$ = 134.599
 Fe = Elastic Buckling Stress: $\pi^2 * E / (Lc/r)^2$ {E3-4} = 182.78 ksi
 Limit = Limit used for Lc/r: $4.71 * (E/Fy)^{0.5}$ = 134.599
 Fcr = Critical Stress: $[0.658^{(Fy/Fe)}] * Fy$ {E3-2} = 33.15 ksi
 Pn_fb = Flexural Buckling Strength: $Fcr * Ag$ {E3-1} = 523.79 Kip

Ver Stiff: Nominal Compressive Strength per AISC 360-16 Sec E4:

Lcx = Effective Length of Member about X-Axis = 102.0000 in
 Lcy = Effective Length of Member about Y-Axis = 102.0000 in
 Lcz = Effective Length of Member about Z-Axis = 102.0000 in
 ho = Dist between flange Centroids: D-Tf = 9.4850 in
 Cw = Warping Constant: From AISC = 2320
 G = Shear Modulus of Elasticity of Steel = 11200.00 ksi
 Fe = $(\pi^2 * E * Cw / Lcz^2 + G * J) * (1 / (Ix + Iy))$ {E4-2} = 209.58 ksi
 Fcr = Critical Stress: $[0.658^{(Fy/Fe)}] * Fy$ {E3-2} = 33.50 ksi
 Pn_FTB = Flexural Torsional Buckling Strength: $Fcr * Ag$ {E4-1} = 529.34 Kip

Ver Stiff: Nominal Compressive Strength per AISC 360-16 E3 & E4:

Pn = Lessor of Pn_fb or Pn_ftb = 523.79 Kip
 Pc = Design Compressive Strength: $Pn / 1.67$ (ASD) = 313.65 Kip

Ver Stiff: Nominal Flexural Strength per AISC 360-16 Sec F6: (Weak Axis)

Mp = Plastic Moment: $Fy * Zy \leq 1.6 * Fy * Sy$ {F6-1} = 1126.80 K-in
 Mn_y = Yielding Nominal Flexural Strength: Mp {F6-1} = 1126.80 K-in
 Flexure Flange Classification Table B4.1b:
 Lambda = Compact criteria taken from AISC 360-16: $Bf / 2Tf$ = 8.150
 Lambda_p = Limiting Width-to-Thk Ratio from Table B4.1b: $0.38 * (E/Fy)^{0.5}$ = 10.859
 Lambda_r = Limiting Width-to-Thk Ratio from Table B4.1b: $1.00 * (E/Fy)^{0.5}$ = 28.577
 FS = Flange Slenderness: Since $\lambda \leq \lambda_p$ = Compact
 Since Flanges are compact the Flange Local Buckling criteria doesn't apply
 Mn = Lessor of Mn_y and Mn_flb = 1126.80 K-in
 Mcy = Design Flexural Strength for ASD: $Mn / 1.67$ {F1} = 674.73 K-in

Ver Stiff: Check Member for Combined Flexure and Axial per Sec. H1-1:

Pr = Required Axial Strength (+ is Compression) = 34.37 Kip
 Mrx = Required Flexural Strength about Strong Axis = 0.00 K-in
 Mry = Required Flexural Strength about Weak Axis = 173.59 K-in
 Pr/Pc = Ratio of Required to Available Axial Strength = 0.1096
 Since $Pr/Pc < 0.2$ then use Eqn H1-1b
 H1-1b = $Pr / (2 * Pc) + (Mrx / Mcx + Mry / Mcy)$ {H1-1b} = 0.312

The section properties for the stack have been modified in the breach region to consider the addition of the vertical stiffeners. The stack stresses and dynamic behavior will all take these modified section properties into account in those analyses.

UR_sb = Shell Buckling Unity Ratio determined previously = 0.309

Since UR_sb <= 1, then the section can distribute the load around the opening, and the circumferential stiffener doesn't need to carry a portion of the load above the opening to each vertical stiffener.

Breach # 1 Ld 8 Circ Stiff: Maintains Roundness by Replacing Cutout Moment of Inertia:

Iy = Stiffener Moment of Inertia for Ring Bending = 2.25 in⁴

Ico = Mom of Inertia out of Plane for Removed Breach: $Ht \cdot Thk^3 / 12$ = 1.00 in⁴

Ireqd = Req'd moment of Inertia for Stiff: Ico = 1.00 in⁴

UR_ring = Ring Bending Mom of Inertia Check: Ireqd / Ico = 0.444

The circumferential stiffening rings are considered as stiffening rings acting on the overall stack to resist ovaling of the stack. The evaluation of ovaling and circumferential stiffener's ability to resist that ovaling is performed elsewhere in the analysis.